

1 RAINFALL INTENSITY-DURATION-FREQUENCY MODELS USING 2 OPTIMIZATION TECHNIQUE FOR ABEOKUTA, SOUTH – WEST NIGERIA

3 4 ABSTRACT

5 The design of water resources engineering control structures is best achieved with adequate
6 estimation of rainfall intensity over a particular catchment. To develop the rainfall intensity,
7 duration and frequency (IDF) models, 25 year daily rainfall data were collected from Nigerian
8 Meteorological Agency (NIMET) Abuja for Abeokuta. The annual maximum rainfall amounts
9 with durations of 5, 10, 15, 20, 30, 45, 60, 90, 120, 180, 240, 300 and 420 minutes were
10 extracted and subjected to frequency analysis using the Excel Optimization Solver wizard.
11 Specific and general IDF models were developed for return periods of 2, 5, 10, 25, 50 and 100
12 years using the Gumbel Extreme Value Type 1 and Log Pearson Type 3 distributions. The
13 Anderson Darling goodness of fit test was used to ascertain the best fit probability distribution.
14 The R^2 values range from 0.973 – 0.993 and the Mean Squared Error, MSE from 84.49 – 134.56
15 for the Gumbel and 0.964 – 0.997 with MSE of 42.88 – 118.68 for Log Pearson Type 3
16 distribution, respectively. The probability distribution models are recommended for the
17 prediction of rainfall intensities for Abeokuta metropolis.

18 Keywords: IDF models, Gumbel Extreme Value Type I, Log Pearson Type 3 distributions,
19 Excel Optimization Solver, goodness of fit test.

20 1. INTRODUCTION

21 The Rainfall Intensity Duration Frequency (IDF) relationship is one of the most commonly
22 used tools for the design of hydraulic and water resources engineering control structures. An IDF
23 model is a mathematical relationship between the rainfall intensity, duration and the frequency
24 (return period). The establishment of such relationship was done as early as 1932 (Bernard,
25 1932). The knowledge of frequency of extreme events like floods, droughts, rainstorm and high
26 winds assisted in planning and design for these extreme events (Hosking and Wallis, 1997). The
27 planning and designing of various water resource projects requires the use of rainfall intensity-
28 duration-frequency (IDF) relationship (El-sayed, 2011). This relationship is determined through
29 frequency analysis of data from meteorological stations. The IDF formulae are the empirical
30 equations representing a relationship among maximum rainfall intensity (as dependent variable)
31 and other parameters of interest such as rainfall duration and frequency (as independent
32 variables). There are several commonly used functions found in the literature of hydrology
33 applications (Chow et al., 1988). Owing to its wide applications, accurate estimation of intensity-
34 duration-frequency relationship has received attention from researchers and scientists from all
35 over the world (Mohammad Zakwan, 2016). All functions have been widely applied in
36 hydrology. In Nigeria, a lot of work has been done in South – East and South – South like the
37 IDF models of Port Harcourt (Nwaogazie & Duru, 2002) and that of Eket in Awka Ibom State

38 (Nwaogazie & Uba, 2001). All these models generated IDF curves that confirms the theory for
39 shorter recurrence periods of 2 to 10 years.

40

41 **2. MATERIALS AND METHODS**

42 **2.1 Area of Study**

43

44 Abeokuta is the capital of Ogun State in South – West Nigeria covering an estimated area of
45 about 40.60 km². It is located at 74m above the sea level and falls within latitude 7° 10' N and 7°
46 15' N and longitudes 3° 17' E and 3° 26' E. Abeokuta lies in the plane which is developed on rocks
47 of the basement complex found in the Savannah zone. The area is properly drained by River
48 Ogun and it is characterized by relatively high temperature with mean annual temperature of
49 30°C and rainfall of 1,185 mm respectively.

50



51

52 Figure 1: Location map of Abeokuta and adjoining cities in South-Western Nigeria

53 Source: Google map (2019)

54 **2.2 Data Collection**

55

56 The major material used for this work is rainfall data comprising of rainfall amount and
57 duration. The twenty five (25) years rainfall data included data ranging from 1986 to 2010. The
58 data were obtained from Nigeria Meteorological Centre (NIMET) office Abuja, Nigeria. This
59 data arrangement involved sorting the data according to years, rainfall intensities and durations.

60 The rainfall intensities selected are the maximum values for each year for all the years analysed.

61

61 **2.3 Data Analysis**

62 The annual maximum data series are obtained by selecting the maximum amount of rainfall for
 63 each year for 5, 10, 15, 20, 30, 45, 60, 90, 120, 180, 240, 300, and 420 durations (minutes) for
 64 the 25 year period.

65 The IDF relation is mathematically expressed as follows:

$$66 \quad I = f(T,d) \quad (1)$$

67 The rainfall amount is converted to intensity (mm/hr) by dividing the amount by the duration
 68 (minutes) then multiplying by 60 as a conversion factor. For instance, given rainfall amount of
 69 54.3mm for 15 minute duration yields an intensity of $(54.3/15) \times 60 = 217.2$ mm/hr

70 Table 1 shows all the intensities for various durations.

71

72 Table 1: Ranked Observed Annual Rainfall Intensities (mm/hr) for different Durations (mins) for
 73 Abeokuta

| Year | Rainfall intensity (mm/hr) | | | | | | | | | | | | |
|------|----------------------------|-------|-------|-------|-------|-------|------|------|------|------|------|------|------|
| | 5 | 10 | 15 | 20 | 30 | 45 | 60 | 90 | 120 | 180 | 240 | 300 | 420 |
| 1 | 421.2 | 271.2 | 217.2 | 186.3 | 140.6 | 112.4 | 88.6 | 59.8 | 54.2 | 40.9 | 32.1 | 25.7 | 18.3 |
| 2 | 381.6 | 270.0 | 189.6 | 174.3 | 129.6 | 93.7 | 84.3 | 59.5 | 44.7 | 36.1 | 30.7 | 24.6 | 17.5 |
| 3 | 336.0 | 257.4 | 180.8 | 166.8 | 129.2 | 89.6 | 82.3 | 59.1 | 44.3 | 30.7 | 27.1 | 21.7 | 15.5 |
| 4 | 330.0 | 248.4 | 180.0 | 162.9 | 125.6 | 86.5 | 70.3 | 58.7 | 44.1 | 29.8 | 25.6 | 20.6 | 14.7 |
| 5 | 295.2 | 231.0 | 178.8 | 142.2 | 124.2 | 86.4 | 67.2 | 54.9 | 41.2 | 29.5 | 23.1 | 20.5 | 14.6 |
| 6 | 289.2 | 221.4 | 171.6 | 135.6 | 116.2 | 86.1 | 64.9 | 54.7 | 41.0 | 27.4 | 22.3 | 18.4 | 13.2 |
| 7 | 233.1 | 210.6 | 169.2 | 135.0 | 94.8 | 85.5 | 64.8 | 44.8 | 33.6 | 27.3 | 22.2 | 17.9 | 12.8 |
| 8 | 223.1 | 190.8 | 167.2 | 134.1 | 90.4 | 85.3 | 64.6 | 43.9 | 33.0 | 22.4 | 20.6 | 17.7 | 12.7 |
| 9 | 196.8 | 171.0 | 165.6 | 128.7 | 89.4 | 84.9 | 64.0 | 43.2 | 32.4 | 22.0 | 20.5 | 16.5 | 12.2 |
| 10 | 195.6 | 168.0 | 154.0 | 126.9 | 85.8 | 82.8 | 63.7 | 43.1 | 32.3 | 21.6 | 19.7 | 16.4 | 11.8 |
| 11 | 187.2 | 165.0 | 147.6 | 125.4 | 83.6 | 78.3 | 62.1 | 42.7 | 32.0 | 21.5 | 17.7 | 15.2 | 11.7 |
| 12 | 186.1 | 152.4 | 140.4 | 124.2 | 82.8 | 77.6 | 58.7 | 42.5 | 31.9 | 21.4 | 16.9 | 14.9 | 11.6 |
| 13 | 181.2 | 147.6 | 131.2 | 123.0 | 82.0 | 63.2 | 58.2 | 39.1 | 29.4 | 21.3 | 16.5 | 14.6 | 10.8 |
| 14 | 170.4 | 146.9 | 127.2 | 122.4 | 81.6 | 60.3 | 47.4 | 38.8 | 29.1 | 21.2 | 16.2 | 13.2 | 10.7 |
| 15 | 167.5 | 144.6 | 120.4 | 115.5 | 77.0 | 57.2 | 44.5 | 37.1 | 28.0 | 20.5 | 16.2 | 13.0 | 10.3 |
| 16 | 162.3 | 140.6 | 112.1 | 110.7 | 73.8 | 55.7 | 44.1 | 35.9 | 27.8 | 19.6 | 16.0 | 12.9 | 9.8 |
| 17 | 161.0 | 124.0 | 112.0 | 95.4 | 70.6 | 55.2 | 42.9 | 33.9 | 26.9 | 19.6 | 15.9 | 12.8 | 9.7 |
| 18 | 149.5 | 117.9 | 107.3 | 92.5 | 67.6 | 54.7 | 42.6 | 32.9 | 26.8 | 19.4 | 15.6 | 12.8 | 9.4 |
| 19 | 149.0 | 117.2 | 96.4 | 90.3 | 63.6 | 53.9 | 41.8 | 32.5 | 24.7 | 18.1 | 14.9 | 12.7 | 9.3 |
| 20 | 137.9 | 111.6 | 94.6 | 88.6 | 60.2 | 51.6 | 41.0 | 31.6 | 23.7 | 17.9 | 14.7 | 12.5 | 9.2 |
| 21 | 135.6 | 105.5 | 90.0 | 78.1 | 59.6 | 51.3 | 38.5 | 29.1 | 23.7 | 17.2 | 14.7 | 12.3 | 9.1 |
| 22 | 119.7 | 102.2 | 89.5 | 74.3 | 56.7 | 45.5 | 38.0 | 28.7 | 22.5 | 17.1 | 14.6 | 12.2 | 8.9 |
| 23 | 117.7 | 101.4 | 80.5 | 73.8 | 56.4 | 43.3 | 37.5 | 28.6 | 22.4 | 16.5 | 14.2 | 12.1 | 8.8 |

| | | | | | | | | | | | | | |
|-------------------------|-------|-------|-------|-------|------|------|------|------|------|------|------|------|------|
| 24 | 117.4 | 98.4 | 78.0 | 72.3 | 50.7 | 43.0 | 35.7 | 28.4 | 21.8 | 15.8 | 14.1 | 11.7 | 8.7 |
| 25 | 115.8 | 98.4 | 77.4 | 66.5 | 49.1 | 41.3 | 35.5 | 27.3 | 21.5 | 15.5 | 13.5 | 11.7 | 8.5 |
| Mean | 206.4 | 164.5 | 135.1 | 117.8 | 85.6 | 69.0 | 55.3 | 41.2 | 31.7 | 22.8 | 19.0 | 15.8 | 11.6 |
| Standard Deviation | 86.8 | 57.5 | 40.7 | 33.6 | 27.3 | 19.7 | 16.1 | 10.9 | 8.7 | 6.5 | 5.3 | 4.1 | 2.8 |
| Coefficient of Skewness | 1.05 | 0.63 | 0.16 | 0.25 | 0.64 | 0.26 | 0.51 | 0.53 | 0.95 | 1.32 | 1.19 | 1.08 | 1.01 |

74 The magnitude of rainfall intensities were obtained using frequency analysis. Two probability
75 distributions namely Gumbel Extreme Value Type I (GEVT-1) and Log-Pearson Type III were
76 used to obtain the magnitude of rainfall intensities for different return periods.

77 **2.4 Gumbel's Extreme Value Type I (GEVT- 1) Distribution**

78 Gumbel distribution is one commonly used probability distribution for obtaining the
79 rainfall intensity values. The rainfall intensity values were obtained using Equation (2)

$$80 \quad X_T = \bar{X} + K_T S \quad (2)$$

81 Where X_T = rainfall intensity values (magnitude of hydrologic event)

82 \bar{X} = mean; K_T = Gumbel's frequency factor; S = standard deviation

83 The Gumbel's frequency factor is obtained using Equation (3).

$$84 \quad K_T = -\frac{\sqrt{6}}{\pi} \left\{ 0.5772 + \ln \left[\ln \left(\frac{T}{T-1} \right) \right] \right\} \quad (3)$$

85 Where T = return period (years)

86 For example, Gumbel frequency factor for a 5 years return period

$$87 \quad K_T = -\frac{\sqrt{6}}{\pi} \left\{ 0.5772 + \ln \left[\ln \left(\frac{5}{5-1} \right) \right] \right\} = 0.719$$

88 The resulting Gumbel K_T values for different return periods as calculated are shown in Table 2.

89

90 Table 2: Gumbel frequency factor for Abeokuta IDF modeling

| | | | | | | |
|---------------|----------|-------|-------|-------|-------|--------|
| Return Period | 2 | 5 | 10 | 25 | 50 | 100 |
| K_T values | -0.16425 | 0.719 | 1.304 | 2.044 | 2.592 | 3.1363 |

91

92 **Calibration of Sherman (1932) IDF model**

93 Sherman's (1932) IDF model is given as

94
$$I = \frac{cT_r^m}{T_d^a} \quad (4)$$

95 Equation (4) is non-linear power law that was calibrated for c, m, a parameters using intensity,
96 duration and return period values in Table 1 and Excel Optimization Solver.

97 **Goodness of fit test**

98 The result in Table 1 was subjected to Anderson-Darling test to ascertain the probability
99 distribution that best fit the rainfall annual maximum amount. This is a nonparametric test of the
100 equality of continuous, one dimensional probability distributions that can be used to compare a
101 sample with a reference probability distribution. GEVT-1 and Log-Pearson Type 3 (LPT-3) best
102 fit the rainfall intensities with significant values of 0.7570 and 0.7538 at 5% confidence level
103 respectively.

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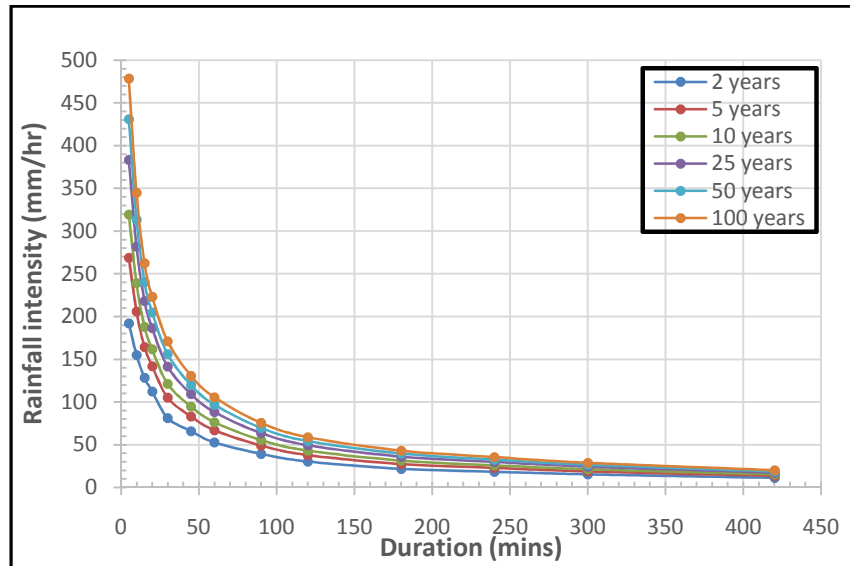
106 **RESULTS**

107 The Anderson-Darling test shows that GEVT-1 and log Pearson Type III best fit the rainfall
108 annual maximum amounts as shown in Table 3

109 The rainfall intensity values are computed by applying Equation (1). Rainfall intensity using
110 GEVT-1 distribution with the mean and standard deviation are obtained from Table 1 For a 5
111 minute duration and 2 years return period, the probability equivalent of rainfall intensity via
112 GEVT-1 is $X_T = \bar{X} + K_T S \Rightarrow X_T = 200.3 + (-0.16425 \times 147.52) \Rightarrow X_T = 200.3 - 24.23 \Rightarrow X_T =$
113 176.07mm/hr

114 Figure 2 shows rainfall intensity distributions and return periods using GEVT-1 distribution.

115



116

117 Figure 2 Intensity Duration Frequency (IDF) curves for GEVT - 1 distribution for Abeokuta.

118 **Calibration of Sherman’s IDF models for specific Return periods**

119 The calibrated Sherman (1932) IDF models for specified return periods are as presented in Table
 120 3. Equally included in the table are coefficient of determination R^2 and mean square error (MSE)
 121 for model performance assessment.

122

123

124

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127

128

129 Table 3: GEVT-1 calibrated IDF Models for different return periods for Abeokuta.

130

| Return Period | IDF Model \pm | Coefficient of Determination (R^2) | Mean Squared Error (MSE) |
|---------------|---|--|--------------------------|
| 2 | $I = \frac{4.857T_r^{6.663}}{T_d^{0.535}}$ | 0.973 | 84.49 |
| 5 | $I = \frac{2.2431T_r^{3.579}}{T_d^{0.569}}$ | 0.985 | 93.05 |
| 10 | $I = \frac{1.676T_r^{2.710}}{T_d^{0.584}}$ | 0.988 | 100.93 |
| 25 | $I = \frac{1.319T_r^{2.075}}{T_d^{0.599}}$ | 0.990 | 112.96 |

| | | | |
|-----|--|-------|--------|
| 50 | $I = \frac{1.197T_r^{1.765}}{T_d^{0.608}}$ | 0.992 | 123.26 |
| 100 | $I = \frac{1.113T_r^{1.540}}{T_d^{0.615}}$ | 0.993 | 134.56 |

131 ±: return period specific IDF models

132 **Evaluation of iterative Equation Solver in Excel**

133 Excel Solver model parameters trial solution for return period (2 year) specific IDF
 134 model has fourteen (14) iterations before convergence (see Table 4). Similarly,
 135 there are thirty-five (35) iterations in the development of the general IDF model
 136 given in Equation (6).

137
 138
 139
 140

Table 4: Trial solution result for Sherman's specific IDF model calibration

| Iteration | C | m | A |
|-----------|----------|----------|----------|
| 1 | 1 | 1 | 1 |
| 2 | 1.461474 | 1.31987 | 0 |
| 3 | 3.546129 | 3.431661 | 0 |
| 4 | 3.825354 | 4.117993 | 0 |
| 5 | 3.830287 | 4.130401 | 0.05 |
| 6 | 4.528795 | 5.887498 | 0.312129 |
| 7 | 4.713106 | 6.348498 | 0.400196 |
| 8 | 4.838772 | 6.614912 | 0.52986 |
| 9 | 4.859924 | 6.669481 | 0.538164 |
| 10 | 4.857193 | 6.663613 | 0.535575 |
| 11 | 4.856903 | 6.662889 | 0.535429 |
| 12 | 4.856903 | 6.662889 | 0.535429 |
| 13 | 4.856903 | 6.662889 | 0.535429 |
| 14 | 4.856903 | 6.662889 | 0.535429 |

141
 142

143 The coefficient of determination is computed from Equation (5) and Table 5

$$144 R^2 = \frac{(\sum_{i=1}^n (y - y_{avg})^2) - \sum_{i=1}^n (y - y_{pred})^2}{\sum_{i=1}^n (y - y_{avg})^2} \quad (5)$$

$$145 R^2 = \frac{(41807.74 - 1098.365)}{41807.74} = 0.973$$

146

147 Calculating the Mean Square Error (MSE) using Equation (6) we have;

$$148 MSE = \frac{\sum_{i=1}^n (y - y_{pred})^2}{n} \quad (6)$$

$$149 MSE = \frac{1098.365}{13} = 84.49$$

150

151

152

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154

155

156 Table 5 Tabular Computation of Coefficient of Determination for 2 year return period

| Intensity | Intensity _{pred} | (I - Ip) ² | (I-lavg) ² |
|-------------------------|---------------------------|-----------------------|-----------------------|
| 192.1498641 | 207.892929 | 247.8440829 | 14668.11 |
| 155.0966423 | 143.436046 | 135.9695073 | 7065.876 |
| 128.463877 | 115.444493 | 169.5043489 | 3297.745 |
| 112.3163251 | 98.9639205 | 178.2867085 | 1703.91 |
| 81.16415026 | 79.6511058 | 2.28930367 | 102.5414 |
| 65.78223051 | 64.1071879 | 2.805767634 | 27.62183 |
| 52.68677814 | 54.9554029 | 5.146658379 | 336.7629 |
| 39.42640188 | 44.2308529 | 23.08274969 | 999.2854 |
| 30.27733462 | 37.9165648 | 58.35783719 | 1661.422 |
| 21.74873497 | 30.517145 | 76.88501435 | 2429.42 |
| 18.13831768 | 26.1605922 | 64.35688805 | 2798.363 |
| 15.11094943 | 23.2144685 | 65.66702178 | 3127.821 |
| 11.13080687 | 19.3872836 | 68.16940809 | 3588.857 |
| Average = 71.038 | | Sum = 1098.365 | Sum = 41807.74 |

157 A general IDF model was also developed. A total of 13 durations multiplied by 6 return periods

158 yields 78 input data point. The entire input data were taken from Table 1.

159 The general IDF model was developed using Excel Optimization Solver. The least

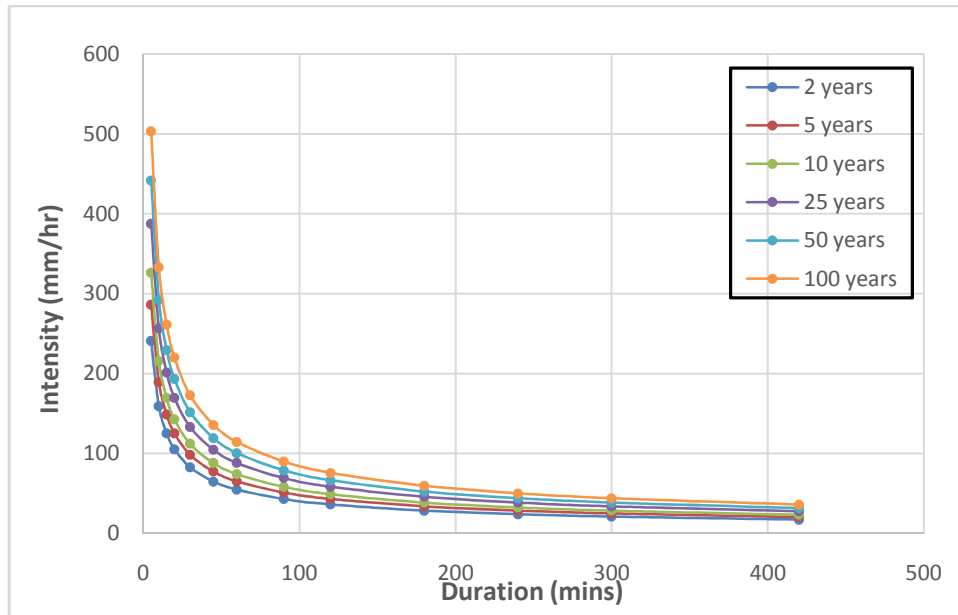
160 squares equations were programmed accordingly.

161

162
$$I = \frac{551.809T_r^{0.188}}{T_d^{0.596}} \quad (6)$$

163 Coefficient of determinant (R^2) = 0.987; Mean Squared Error = 147.70 mm/hr

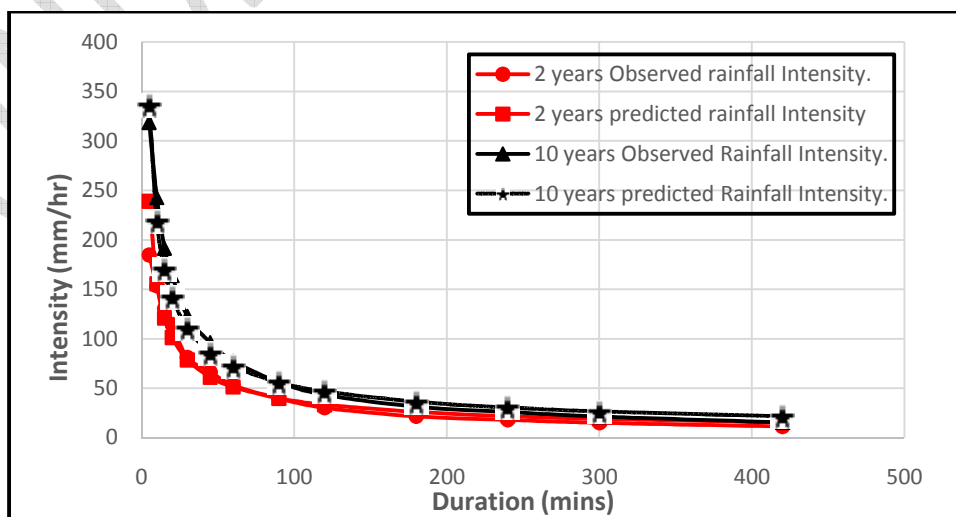
164 The plot of the predicted intensity values of Equation (6) is as given in Figure 3.



165
166 Figure 3 Intensity Duration Curve for Gumbel Extreme Value Type I IDF general model for
167 Abeokuta.

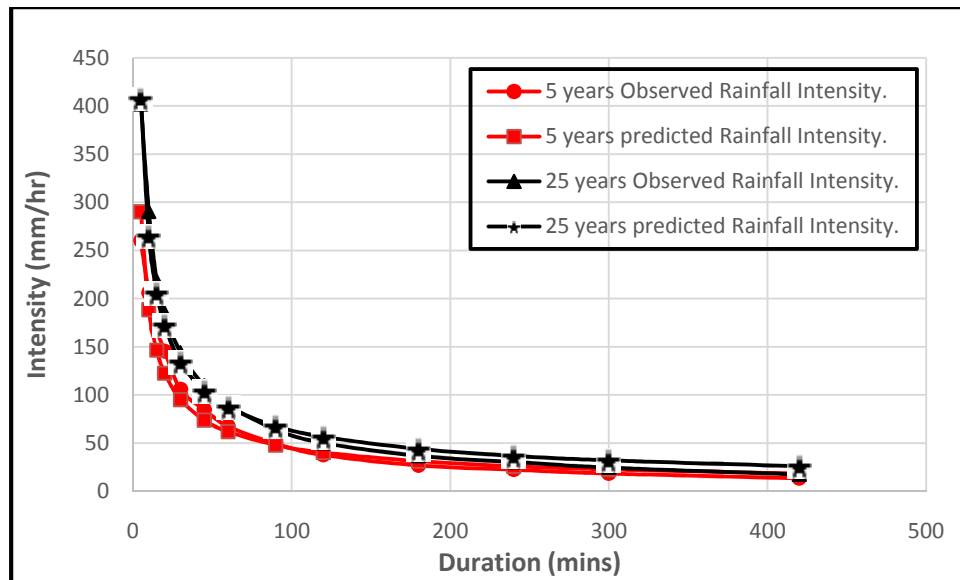
168
169
170 **Comparison of Observed and Predicted Rainfall Intensities**

171
172 This model enables one to predict the intensity of rainfall of any duration and any return period.
173 The verification of the developed model is carried out by plotting the observed and predicted
174 intensities on the same graph as shown in Figures 4 to 6.



176 Figure 4 Observed rainfall intensity against predicted rainfall intensity for 2 and 10 year return
177 periods for Log-Pearson Type-3 distribution

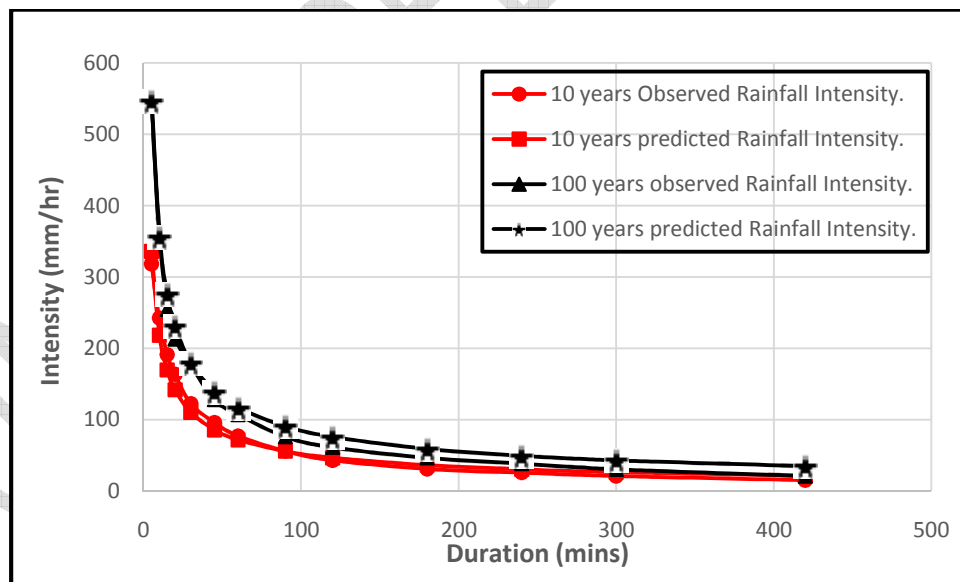
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180 Figure 5 Observed rainfall intensity against predicted rainfall intensity for 5 and 25 year return
181 periods for Log-Pearson Type-3 distribution

182



183

184 Figure 6 Observed rainfall intensity against predicted rainfall intensity for 10 and 100 year return
185 periods for Log-Pearson Type-3 distribution

186

187 **Comparison of Regression Approach and Excel Optimization Solver results for model**
188 **parameters, R^2 and MSE**

189
190 Table 6 (an extension of Table 5) clearly shows the result from Excel Optimization Solver option
191 is more reliable than the normal regression method, the conventional simultaneous solution using
192 matrix i.e. Gauss elimination, inverse or determinant approach.

193
194 Table 6 Results from regression approach and excel solver optimization approach (GEVT-1, 2
195 year return period)

| Method | C | m | A | R^2 | MSE |
|------------|-------|-------|-------|-------|--------|
| Regression | 65.31 | 3.532 | 0.675 | 0.897 | 330.18 |
| Excel | 4.857 | 6.663 | 0.535 | 0.973 | 84.49 |

197 **CONCLUSION**

198 The developed models for GEVT-1 and Log Pearson Type III are in agreement
199 with PDF theory which shows higher intensity occurring at lower duration and
200 lower intensity at higher duration. The prediction of rainfall intensity with the
201 PDFs showed a good match with observed intensity values. The log Pearson Type
202 III model ranked as the best with respect to MSE 54.22 and R^2 0.998 in the return
203 period specific model. The comparison of PDF and non-PDFs shows that the
204 former has lesser MSE value than the later; 84.49 and 330.18 respectively.

205

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UNDER PEER REVIEW